

HIGH-DENSITY COASTAL ARMOUR UNITS FOR MANAGING INCREASED RISK WITH CLIMATE CHANGE

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Ron Cox, Ben Modra, Francois Flocard and Paul Ryan

Presentation – Ron Cox

Honorary Associate Professor Water Research Laboratory UNSW Sydney

PIANC Australia & New Zealand Board Member

Member PIANC international EnviCom, PTGCC, CoCom

Chair Paper Selection Committee – PIANC COPEDEC conferences

Overview

Implications of Climate Change

-wave climate and sea level rise

Upgrading existing structures

Low carbon, high density GPC CAUs

Field trials

Further research

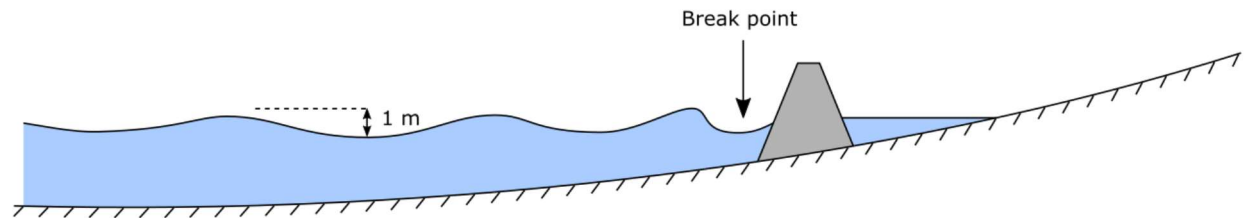
Future opportunities



Photo: Frank Redward

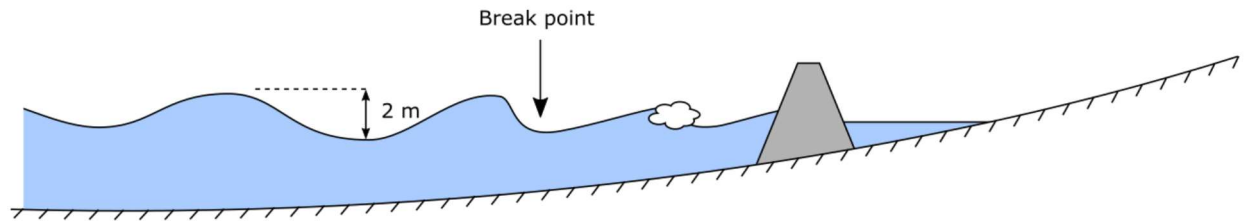
Depth-limited wave conditions

Wave height dependent on water depth

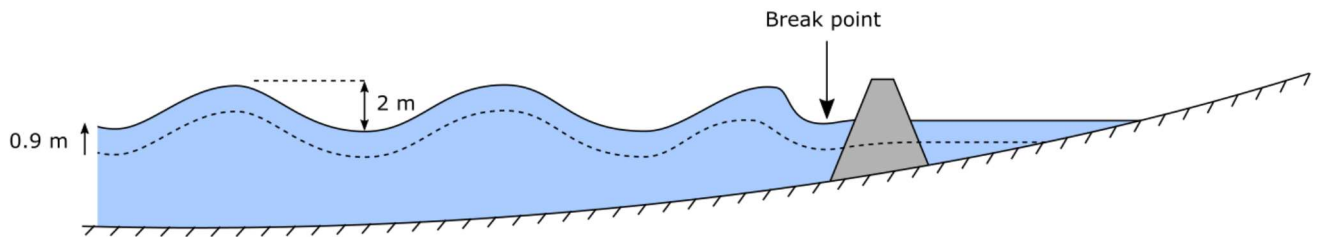


2016

Larger waves break further offshore



Sea level rise brings larger waves close to shore



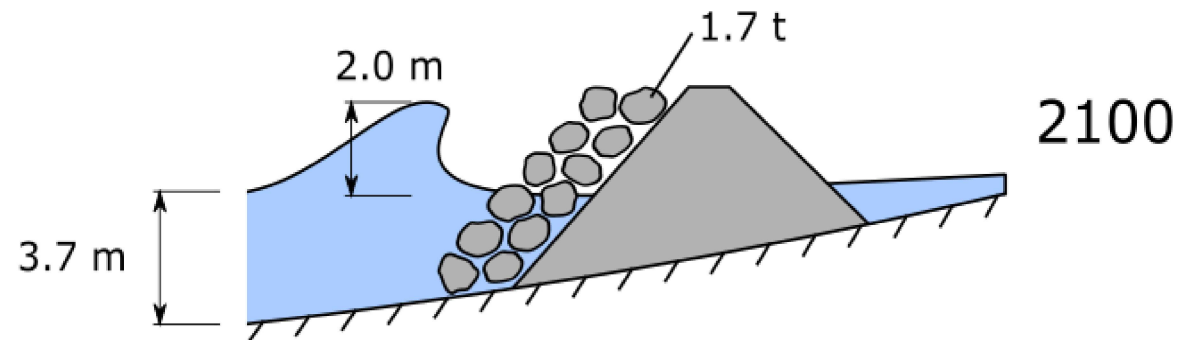
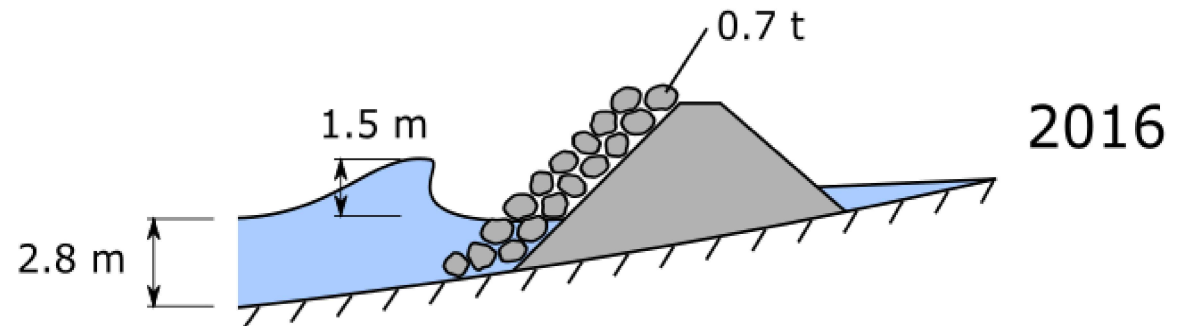
2100

Design armour size

Case study: Tweed Heads
breakwater

With sea level rise:

- wave height increases 33%
- armour size increases 240%



Breakwater stability equation – Hudson

$$M = \frac{\rho_a H^3}{K_d \Delta^3 \cot \alpha}$$

M is the mass of the rock armour or CAU

H is the incident wave height

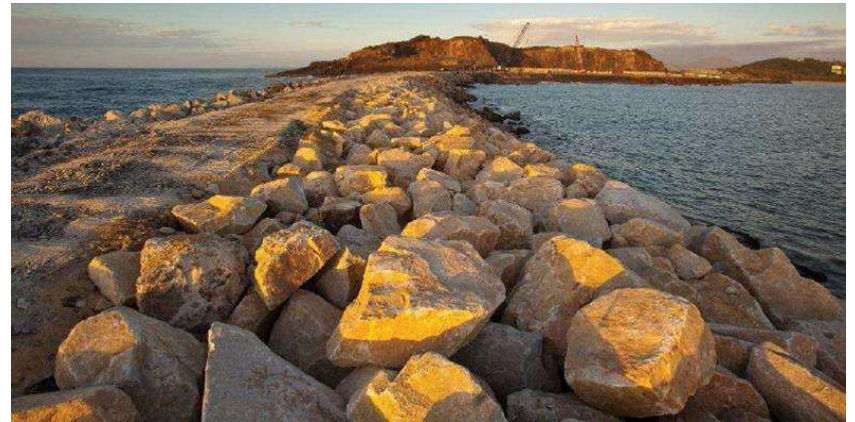
α is the structure slope

ρ_a is the rock armour or CAU density


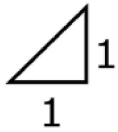

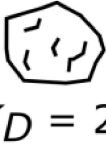


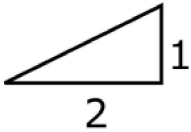



ρ_w is the water density

$\Delta = \frac{\rho_a}{\rho_w} - 1$ is the relative submerged density

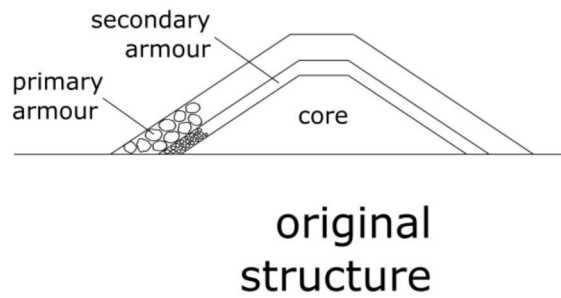
K_D is the Hudson damage coefficient



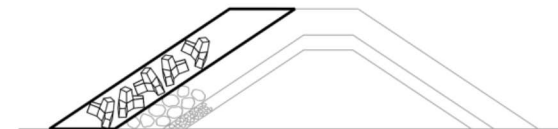
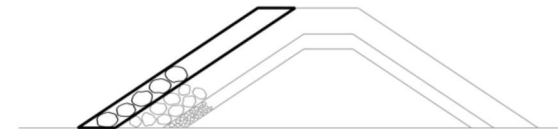
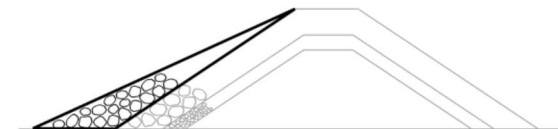
Parameters affecting armour stability

	H wave height	θ seaward slope	M armour mass	K_D stability coefficient	ρ, Δ armour density
Less stable				 $K_D = 2$	
More stable				 $K_D = 7$	

Upgrade options - rock



upgrade options

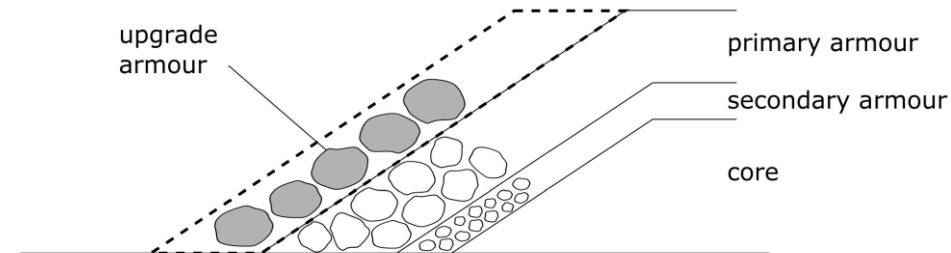


Previous work – Alice Harrison

Single layer armour was effective

Harrison, A., and Cox, R. Physical and economic feasibility of rubble mound breakwater upgrades for sea level rise, Coasts & Ports Conference 2015.

Rock structures depend on mass for stability (not interlocking)



Alice Harrison: Rock armour

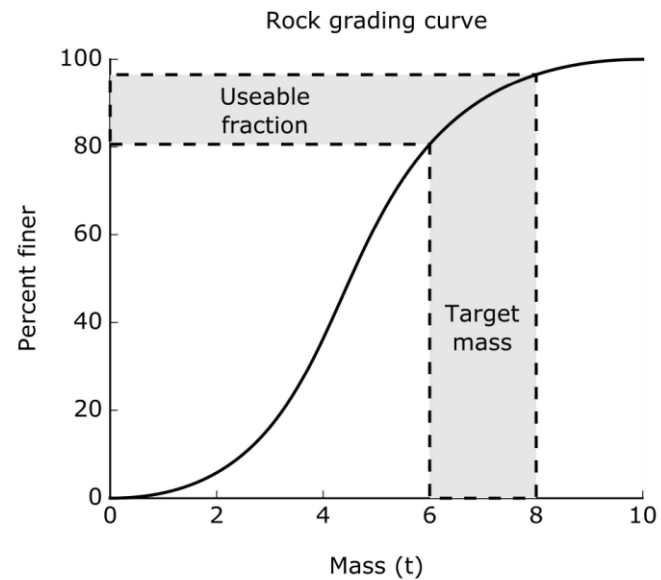
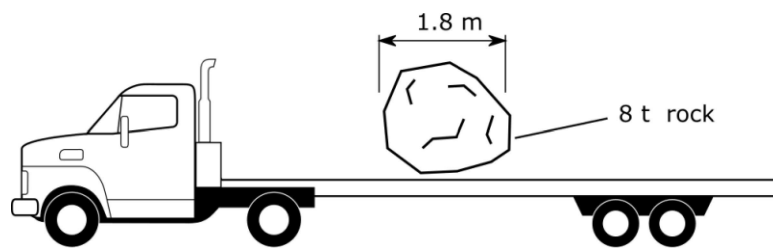
Physical modelling at WRL – upgrading for climate change



Rock size limit

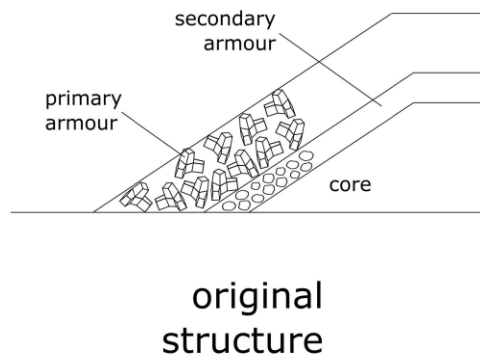
Current economic size limit: 5 – 8 t

- Lack of nearby quarries
- Larger rock target reduces total yield
- Transportation challenges

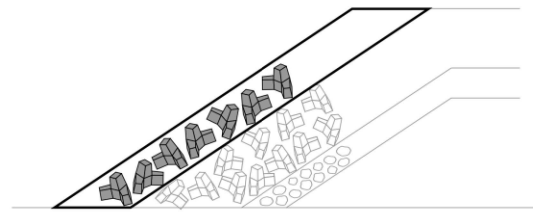
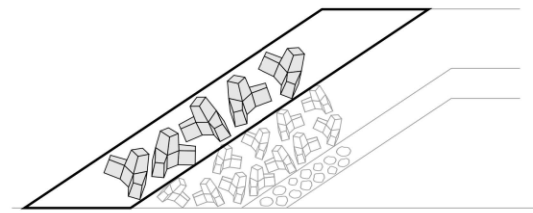


CIRIA, 2007

Upgrade options – concrete armour units



upgrade options



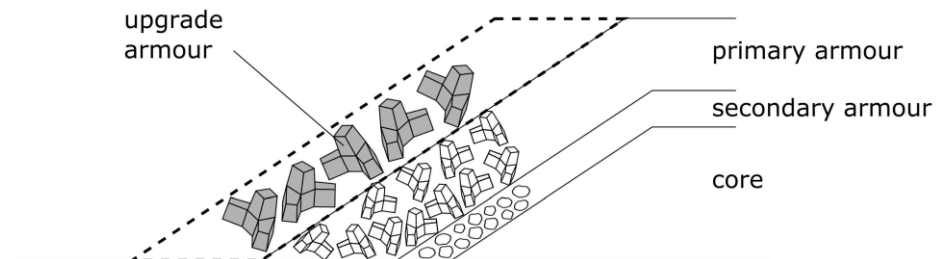
Previous work – Calvin Li

Poor interlocking between different-sized units

Single layer armour upgrade could be effective, (but):

- Stability sensitive to placement density
- Difficult to construct

Li, C., and Cox, R. Stability of Hanbars for upgrading of breakwaters with sea level rise. Coasts and Ports Conference 2013



Single layer Hanbar upgrade not recommended

Calvin Li: Hanbar armour

Physical Model

Scale 1:51.2

Model seaside slope 1:2

Bathymetry slope 1:50

1000 random waves

Hs measured with 3 probe array



Constructability considerations











Current random placement cannot achieve the required density

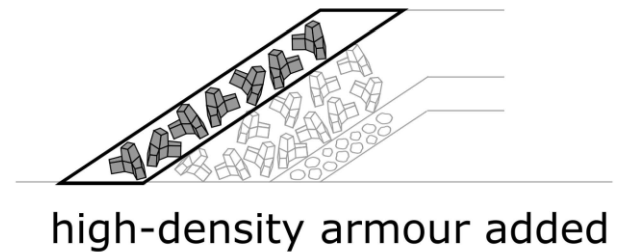
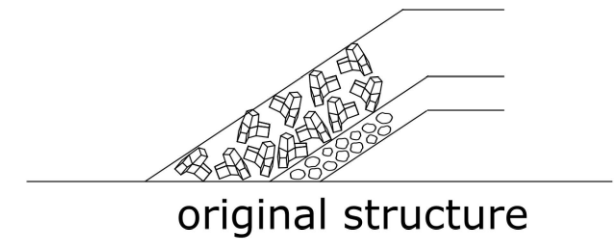
May be achieved by computer aided construction systems



Source: NSW Public Works

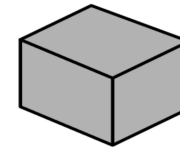
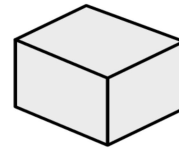
Increase armour density

	H wave height	θ seaward slope	M armour mass	K_D stability coefficient	ρ, Δ armour density
Less stable				 $K_D = 2$	
More stable				 $K_D = 7$	



Hudson equation $M = \frac{\rho_a H^3}{K_D \Delta^3 \cot \theta}$

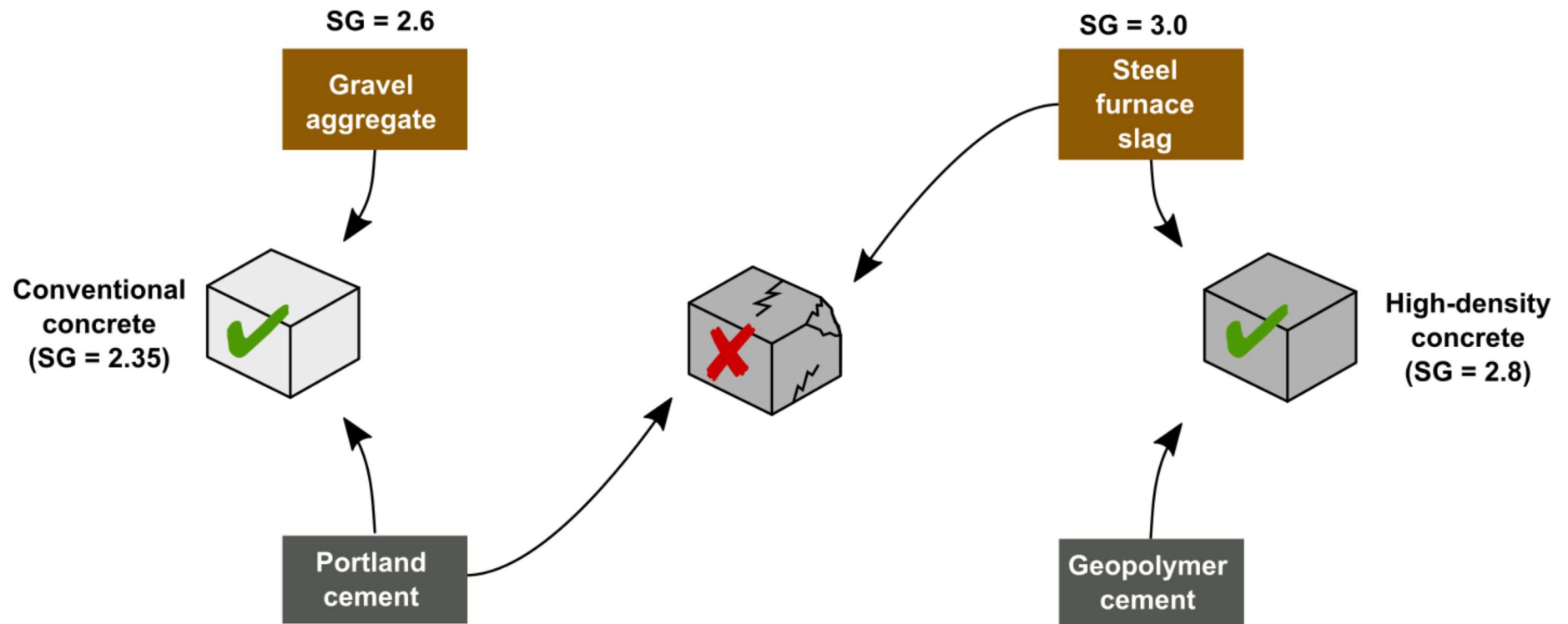
Importance of Δ^3 = submerged relative density cubed



	Conventional concrete	High-density concrete	High - density/ conventional
SG	2.3	2.6 to 2.8	1.1 to 1.2
Δ	1.3	1.6 to 1.8	1.2 to 1.4
Δ^3	2.2	4.1 to 5.8	1.9 to 2.6

$$\Delta = \frac{\rho_a}{\rho_w} - 1$$

High-density GPC concrete (50% less carbon than OPC)



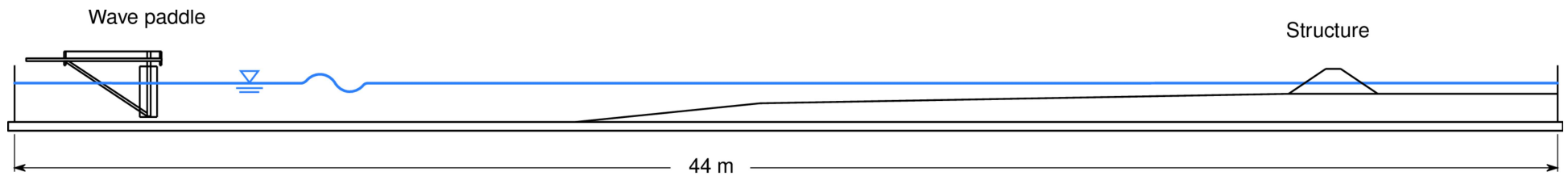
Experimental setup

Facility: 1.2 m wide wave flume

Scale: 1:33

T_p : 9, 11, 13 s

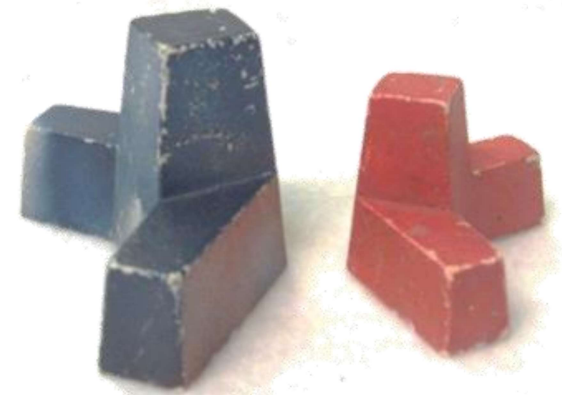
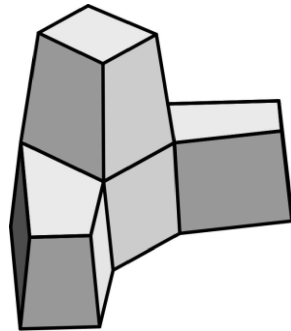
$H_{sig} (max)$: 5.5 m (depth limited)



Present investigation

SG: 2.35, 2.8

Unit: Hanbar



	Conventional concrete	High-density concrete
Mass (t)	20	10.5
Height (m)	2.7	2.0
Density (kg/m ³)	2300	2730

High density Hanbars

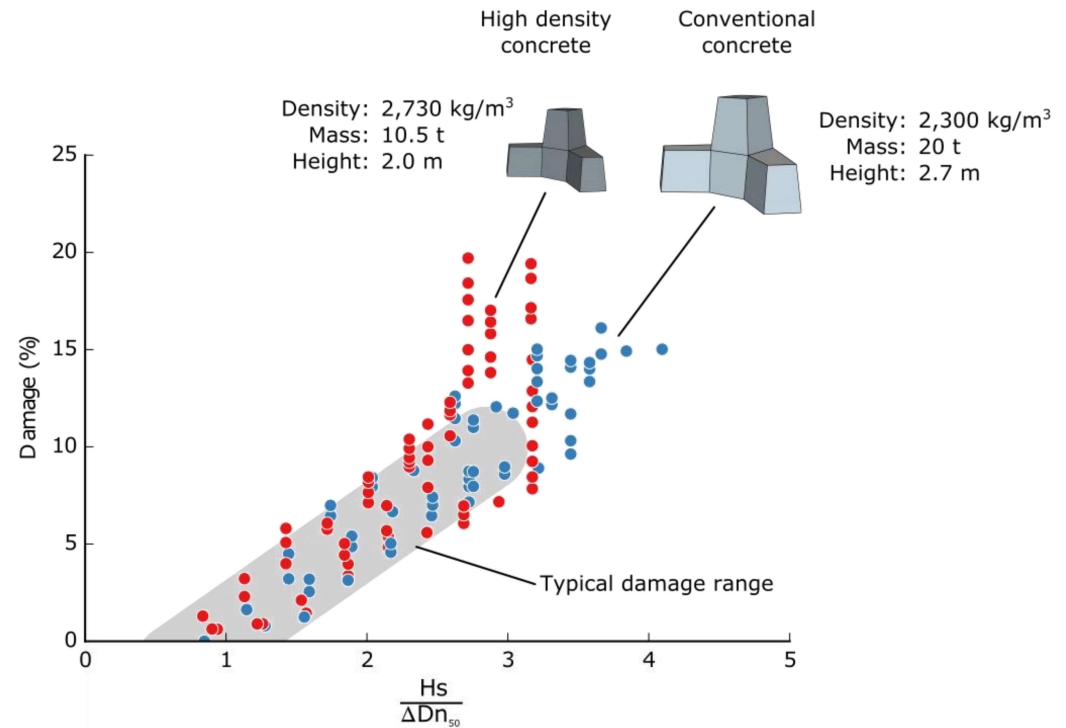
Hudson's equation remains valid
for moderate levels of damage



0% damage



11% damage



Applications

1. Retrofit existing structures

- Interlock with existing armour
- Increase stability

2. Build new structures

- Reduce concrete requirements
- Reduce carbon cost
- Reduce footprint



Development steps

- Improve GPC concrete workability at laboratory scale
- Conduct field trial to prove batching, transport, casting and placement at scale
- Monitor for concrete durability in harsh marine environment
- Further wave flume testing of different CAUs
- Further field scale seawall installations



High Density Geopolymer Concrete field trials

2 years of laboratory mix design to improve strength and workability

Port Kembla significantly damaged in July 2016 East Coast Low storm

Trial to prove batching, transport, casting at scale and placement on Northern Breakwater at Port Kembla– 13 x 18 t GPC Hanbars placed on 2 July 2018

Existing Ordinary Portland Cement OPC Hanbars 16t, 7m³, 2.3 t/m³

Trial Low Carbon HD GPC Hanbars (same mould) 18t, 7m³, 2.6 t/m³

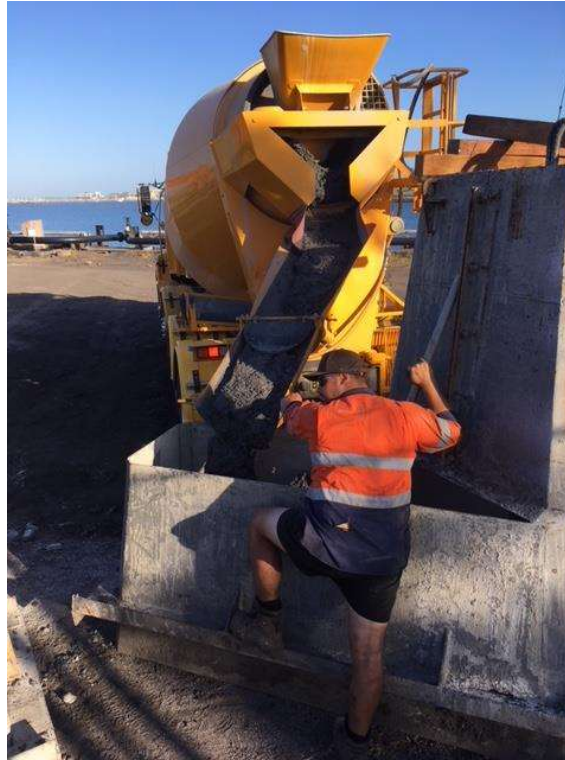
Δ^3 = submerged relative density cubed = $\{(2.6-1)/(2.3-1)\}^3 = 1.9$

GPC has 50% lower carbon footprint than the OPC

Ongoing monitoring of durability in harsh marine environment

Trial batch and casting – 11 April 2018 – 3 HD GPC 18 t Hanbars





Two further GPC batches and casting of extra 10 GPC units - placement of 13 x 18 t Hanbars on 2 July 2018 - Port Kembla Northern Breakwater



Performance Monitoring

May 2021 - Detailed monitoring with core samples and laboratory analysis - strength retained at 40 Mpa after 3 years – some durability issues due to poor quality of some fly ash and slag source material

February 2023 – site inspection revealed some brown staining, but no cracking of units even after rough placement

Northern Breakwater with 16 tonne OPC Hanbars has performed well - showing little damage even though exposed to storms of 5, 10 and 18 year ARI in the period since 2018.

The trial low carbon 18 tonne high density GPC Hanbar CAUs have also performed well with some superficial discoloration, no structural breakage and some displacement caused by the poor initial placement.

Further research and scale installation

The trial has demonstrated that low carbon high density GPC Hanbar CAUs have progressed from a laboratory scale concept to a real opportunity for full scaled application – for upgrades or new structures.

Next step in “proof of concept” - construction of a small seawall section in a relatively sheltered environment with relatively small 2 to 5 tonne low carbon high density GPC Hanbar CAUs.

Ongoing laboratory wave flume stability testing of different CAUs to the simpler Hanbar is needed. This should include application of high density GPC CAUs to new construction and upgrading of existing structures for increased wave exposure with climate change.

Questions

Ron Cox

r.cox@unsw.edu.au